

Module 205

SCS Runoff Equation.EDC

Engineering Hydrology Training Series

Module 205-SCS Runoff Equation

Module Description

Objectives

Upon completion of this module, the participant will be able to:

- Describe water balance.
- Derive the SCS Runoff Equation.
- Explain the application of the SCS Runoff Equation.
- Explain Runoff Curve Numbers.
- List limitations on the use of the SCS Runoff Equation.
- Perform at ASK Level 3 (perform with supervision).

Prerequisites

Modules 103-Runoff Concepts and IOS-Runoff Computations

Contents

This module discusses the concepts assumed in the rationale for the basic equation. Parameters are defined and explained. The runoff equation is developed step by step, and attention is given to correctness in use of the parameters as well as the algebra.

Introduction

The NRCS method of estimating direct runoff from rainfall was developed by SCS hydrologists during the early 1950's. The primary uses for such estimates are to establish safe limits for hydrologic design and to compare the effects of alternative conservation measures in a watershed. Runoff needs for NRCS work include volume estimates for "short" duration detention and peak discharge estimates.

Watersheds that NRCS are generally concerned with are small and ungaged (no streamgauge records available), thus any runoff procedures should be appropriate for these conditions. Data that are generally available or that can be easily obtained for a watershed include:

- Watershed location
- Published rainfall-frequency data
- Land use and cover information
- Soils information

Considering the needs of NRCS and the data that were available at the time the model was developed, an event model with generalized runoff parameters was used. Streamflow data for small watersheds were available from the Agricultural Research Service and the U.S. Geological Survey. This information formed the data base for the runoff curve numbers that would be used in the SCS runoff equation.

Water Balance

A water balance accounts for all water that enters and leaves a system. For our needs, the system is a watershed and the balance is:

$$\text{Rainfall} = \text{Runoff} + \text{Losses}$$

Solving the water balance for runoff yields:

$$\text{Runoff} = \text{Rainfall} - \text{Losses}$$

Runoff

Runoff is classified by type as it flows through the watershed. The runoff types and their characteristics are:

- Channel runoff-Rainfall that falls on the watercourse. Generally this is a negligible quantity.
- Surface runoff-Generated when the rainfall rate exceeds the infiltration rate.
- Subsurface flow-The horizontal movement of infiltrated water in the soil. Subsurface flow may reappear as surface runoff shortly after rainfall (through seeps or springs). It can also be referred to as "quick return flow" or "interflow."
- Direct runoff-A collective term that includes all the above runoff types.
- Baseflow-A fairly steady release of water from natural or manmade storage areas, such as lakes, swamps, or maybe underground aquifers. It is the flow that lingers on after the immediate effects of a runoff event have occurred.

Rainfall

Rainfall as used in the SCS runoff equation is considered an event amount. Data that were available for development of the procedure were not for events, but for calendar days or 24-hour periods. During times of thunderstorms, either of these amounts could include more than one actual event.

Losses

Losses include rainfall interception, soil infiltration, surface storage, and evaporation.

- *Interception-Rainfall* that does not reach the ground because it has contacted something (generally vegetation or buildings).
- *Infiltration-Water* that reaches the ground and enters the soil. Infiltrated water may become part of subsurface flow or the ground water table.
- *Surface storage-Water* that reaches the ground and is collected in depressions (low spots) along the flow path or in a closed basin. Water in surface storage must either infiltrate or evaporate.
- *Evaporation-Water* that goes back to the atmosphere. It is usually a negligible amount.

Water Balance Equation

Expressing the water balance as an equation:

$$Q = P - (I_a + F)$$

where:

Q = direct runoff (ins) P = rainfall (ins)

I_a = sum of all losses before the beginning of runoff (ins)

F = retention after runoff begins (ins)

The water balance equation has four parameters. During development of the SCS runoff equation, Q and P were measured in the field. I_a could have been computed but a generalized relationship was developed from data. F was solved for and generalized as the variable S (which will be explained later).

For the application phase of the equation; Q is the quantity that is unknown. P can be obtained from either actual measurements or generalized rainfall-frequency analysis. I_a and F are functions of S and are fitted based on the development work. S was created to express I_a and F in terms of one equation fitting parameter and represents the hypothetical limit of storage. It is defined as the potential maximum retention after runoff begins. S lumps all variation in the runoff response because of land use, soils, soil moisture, or rainfall pattern, duration, or intensity, plus any other variation into one variable.

Because all of the losses are grouped together and not defined by amount and source, the model to be developed is called a "lumped system" model.

SCS Runoff Equation

To transform the water balance equation to what is called the SCS runoff equation, two assumptions were made:

Assumption # 1

The ratio of the percent water that has been retained to the maximum potential retention is the same as the ratio of the percent water that ran off to the maximum rainfall available for runoff. This assumption is expressed as:

$$F/S = Q/(P-I_a)$$

Where F is the amount of rainfall retained (after runoff begins); S is the maximum potential retention (after runoff begins); Q is the amount of runoff, and P-I_a is the maximum rainfall available for runoff.

At the limit where P is exceptionally large:

$$Q/(P-I_a) \ll 1, \quad \text{and} \quad F/S \ll 1.$$

When no runoff occurs, P = I_a, both F and Q are zero. Therefore:

$$F/S \ll 0, \quad \text{and}$$

$$Q/(P-I_a) \ll 0$$

Because these ratios are equal in their extremes, they are assumed to exhibit similar characteristics throughout their range. To get the generalized rainfall-runoff relation, solve both the water balance and the assumption #1 ratios for F and equate, thus eliminating F.

Water balance $F = (P - I_a) - Q$

Assumption #1 $F = QS/(P - I_a)$

Equate $(P - I_a) - Q = QS/(P - I_a)$ $(P - I_a)[(P - I_a) - Q] = QS$

Multiply by $(P - I_a)$

Group Q on one side $(P - I_a)^2 = Q(P - I_a + S)$

Solve for Q $(P - I_a)^2 / (P - I_a + S) = Q$

Assumption #2

I_a can be expressed as a function of S. If I_a is expressed as a function of one of the other equation parameters, then the runoff equation is greatly simplified because only rainfall and one fitting parameter (S) are needed to solve for runoff. The relationship that is used by NRCS is:

$$I_a = 0.2 S.$$

Substituting for I_a in the generalized runoff equation produces

$$Q = (P - 0.2 S)^2 / (P - 0.2 S + S)$$

and collecting terms produces the SCS runoff equation:

$$Q = (P - 0.2 S)^2 / (P + 0.8 S)$$

During development, I_a was measured at sites that had both continuous rainfall and runoff records. With both of these known, a solution for S could be found from the water balance and assumption # 1. A log plot of S versus I_a for many such events produced the scatter diagram in figure 1. The mean value of the relation ($I_a = 0.2 S$) was selected. The data used to develop figure 1 were primarily from small agricultural watersheds and represent a wide range of storm conditions.

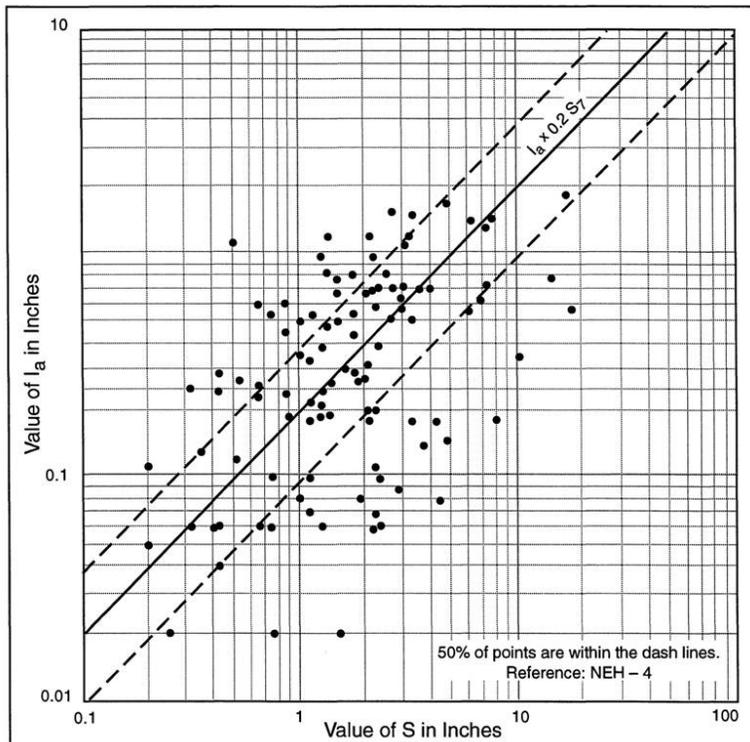


Figure 1. Relationship of I_a and S based on experimental watershed data. Reference: NEH-4.

Runoff Curve Numbers (CN)

The potential maximum retention after runoff begins (S) has a range of values from 0 to infinity. S also requires the use of several decimal places to achieve any type of accuracy in its practical range (0 to 15). To use a more convenient value, a new parameter called curve number (CN), was established. The relationship between CN and S is an inverse relationship and the range of CN is limited by 0 and 100. Its practical range is 40 to 98. CN is also used only as an integer value.

The relationship between CN and S is:

$$CN = 1000 / (S + 10) \quad \text{or} \quad S = 1000 / (CN - 10)$$

Table 1 is a solution of the above equations for the range of curve numbers 40 to 98.

Tables that give the CN to use in the runoff equation for various cover types and hydrologic soil groups are available. The cover types describe not only what is on the land, but in some cases its condition from a hydrologic standpoint (good, fair, or poor). The soils for the site are classified into one of four hydrologic soil groups, depending on the soils' ability to infiltrate water. The soil groups are called A, B, C, and D, which indicate the greatest infiltration capacity to the least, respectively. Cover/hydrologic soil group/CN data are given in table 9.1 in the National Engineering Handbook, Section 4 (Hydrology); Tables 2-3a through 2-3d in the Engineering Field Manual and Table 2-2a through 2-2d in Urban Hydrology for Small Watersheds (TR-55). The tables have been expanded over time and the most complete table is in TR55. Table 2 is a sample.

Curve Number (CN)	Potential Maximum Retention (5)	Curve Number (CN)	Potential Maximum Retention (5)
40	15.000	70	4.286
41	14.390	71	4.085
42	13.810	72	3.889
43	13.256	73	3.699
44	12.727	74	3.514
45	12.222	75	3.333
46	11.739	76	3.158
47	11.277	77	2.987
48	10.833	78	2.821
49	10.408	79	2.658
50	10.000	80	2.500
51	9.608	81	2.346
52	9.231	82	2.195
53	8.868	83	2.048
54	8.519	84	1.905
55	8.182	85	1.765
56	7.857	86	1.628
57	7.544	87	1.494
58	7.241	88	1.364
59	6.949	89	1.236
60	6.667	90	1.111
61	6.393	91	0.989
62	6.129	92	0.870
63	5.873	93	0.753
64	5.625	94	0.628
65	5.385	95	0.526
66	5.152	96	0.417
67	4.925	97	0.309
68	4.706	98	0.204
69	4.493		
70	4.286		

Table 1. Curve numbers and equivalent potential maximum retention values.

Cover description			Curve numbers for hydrologic soil group-			
cover type	Treatment	Hydrologic condition"	A	BC	D	
Fallow	Bare Soil		-77	86	91	94
	Crop residue cover (CR)	Poor	76	85	90	93
Good		74	83	88	90	
Row crops	Straight row (SR)	Poor	72	81	88	91
		Good	67	78	85	89
		Poor	71	80	87	90
	SR + CR	Good	64	75	82	85
		Poor	70	79	84	88
	Contoured (e)	Good	65	75	82	86
		Poor	69	78	83	87
	C+CR	Good	64	74	81	85
		Poor	66	74	80	82
	Contoured & terraced (C& T)	Good	62	71	78	81
Poor		65	73	79	81	
Small grain	SR	Poor	65	76	84	88
		Good	63	75	83	87
		Poor	64	75	83	86
	SR + CR	Good	60	72	80	84
		Poor	63	74	82	85
	C	Good	61	73	81	84
		Poor	62	73	81	84
	C+CR	Good	60	72	80	83
		Poor	61	72	79	82
	C&T	Good	59	70	78	81
Poor		60	71	78	81	
C&T + CR	Good	58	69	77	80	
	Poor	66	77	85	89	
Close-seeded or broadcast legumes or rotation meadow	SR	Good	58	72	81	85
		Poor	64	75	83	85
C	Good	55	69	78	83	
	Poor	63	73	80	83	
C&T	Good	51	67	76	80	

Table 2. Sample curve numbers. Reference: TR-55

Average runoff condition, and $I_a=0.2S$.

'Crop residue cover applies only if residue is on at least 5% of the surface throughout the year. "Hydrologic condition is based on a combination of factors that affect infiltration and runoff, including (a) density and canopy of vegetative areas, (b) amount of year-round cover, (c) amount of grass or close-seeded legumes in rotations, (d) percent of residue cover on the land surface (good ~ 20%), and (e) degree of surface roughness.

Poor: Factors impair infiltration and tend to increase runoff.

Good: Factors encourage average and better than average infiltration and tend to decrease runoff.

The CN's published in these tables are for an average runoff condition. While the exact conditions that are considered average are not defined, some items that can cause runoff conditions to vary are:

- Rainfall amount,
- Rainfall pattern,
- Maximum rainfall intensity,
- Rainfall duration,
- Antecedent soil moisture, and
- Air temperature.

When the curve number tables were originally developed, sample watershed P and Q data pairs were plotted to produce a scatter of points. The CN for each event was determined by "reverse engineering" the runoff equation to solve for S and then converting S to CN. A median curve number was determined and plotted along with the sample data. This median CN represents the average runoff condition. The range of the remaining CN's were noted and later smoothed mathematically. This range is considered to define the limiting values for a specified average runoff condition CN. Table 3 lists the limiting CN values.

Theoretically, any value within the range could be expected to occur in a watershed for some event, some time. Thus if rainfall and runoff data are available for an event, an event CN could be computed. This event CN should fall within the range listed for the appropriate average condition CN. Also, any CN within the range could be used for watershed analysis and design. Any variation from the average condition CN should be documented. In most situations the average condition value is adequate.

Average Condition CN	Lower Limit CN	Upper Limit CN		Average Condition CN	Lower Limit CN	Upper Limit CN
40	22	60		70	51	85
41	23	61		71	52	86
42	24	62		72	53	86
43	25	63		73	54	87
44	25	64		74	55	88
45	26	65		75	57	88
46	27	66		76	58	89
47	28	67		77	59	89
48	29	68		78	60	90
49	30	69		79	62	91
50	31	70		80	63	91
51	31	70		81	64	92
52	32	71		82	66	92
53	33	72		83	67	93
54	34	73		84	68	93
55	35	74		85	70	94
56	36	75		86	72	94
57	37	75		87	73	95
58	38	76		88	75	95
59	39	77		89	76	96
60	40	78		90	78	96
61	41	78		91	80	97
62	42	79		92	81	97
63	43	80		93	83	98
64	44	81		94	85	98
65	45	82		95	87	98
66	46	82		96	89	99
67	47	83		97	91	99
68	48	84		98	94	99
69	50	84				
	70	85				

Table 3. Curve number ranges for average condition curve numbers.

Application Of The SCS Runoff Equation

Before the SCS runoff equation can be used, the land area, cover description, hydrologic soil group, and rainfall frequency must be determined.

Land area and cover description

Field observation, aerial photographs or a combination of these topographic maps, can be used to determine the land area and cover description.

Hydrologic soil group

The published soil survey for the site can be used to determine appropriate soil names. These names, in turn, are referred to on the applicable table where the hydrologic soil groups are found. Tables, such as table 7.1 in NEH Section 4, Exhibit 2-1 in the EFM; or Exhibit A-I in TR-55 should be used. Table 4 shows a sample soil name/hydrologic soil group table.

Curve number

Using the cover description and the hydrologic soil group, a curve number can be determined from the CN tables. If more than one cover or hydrologic soil group are present, a weighted CN must be determined to represent the entire area. CN values are weighted by the applicable area to which they pertain.

Lamkin	B	Lariat	B	Lavina	D	Leetonla	C	Lew	B	Laped	D	Lindale	C
Lamo	C	Larim	B	Lavan	C	Leevan	C	Lewbeach	C	Lapeer	B	Lindell	C
Lamdille	B	Larimer	B	Lawal	B	Lefor	B	Lewdlac	D	Lapham	A	Linden	B
Lamondl	B	Larioscamp	D	Lawen	B	Legall	B	Lewis	D	lapine	A	Linder	B
Lamonl	C	larkin	B	Lawet	BID	Legault	D	Lewisberry	B	Laplatta	C	Lindley	C
Lamont	B	Larkson	C	Lawet,	B	Leggett	C	Lewisburg	C	Lapon	D	Lindrith	B
Lamonta	D	Larmlne	D	Saline-Alkali		Legler	B	Lewiston	C	Laporte	D	Lindside	C
Lamoose	D	Laroque	B	Lawler	B	Leore	B	Lewlsville	B	Laposa	C	Lindstrom	B
Lamotte	B	Larose	D	Lawndale	B	Le ew	C	Lewkalb	C	lapwai	B	Lindy	C
Lamoure	C	Larrupln	B	Lawnwood	BID	Lehigh	C	Lex	B	Larand	B	Line	B
Lampasas	D	Larry	D	Lawnwood,	D	Lehmans	D	Lexington	B	Larchmont	B	Lineville	C
Lamphier	B	Larry, Drained	C	Depressional		Lehr	B	Lexton	B	Lardell	C	Linanore	B
Lampshre	D	Larson	D	Lawrence	C	leicester	C	Leyba	B	Laredo	B	Lin art	A
Lamson	BID	Larton	A	Lawrenceville	C	Leldl	C	Leyden	C	Lares	C	Lininger	C
Lanark	B	Larue	A	Lawshe	D	Leighcan	B	Libblngs	D	Largo	B	Lanyon	C/D
Lancaster	B	Larush	B	Lawson	C	Lellehua	B	Libeg	B	Substratum	B	Lap	D
Lance	B	Larvie	D	Lawther	D	Leisy	B	Liberal	D	Laugenour, Drained	C	Laparita	C
Land	C	Las	C	Lawton	C	Lela	D	Libory	A	laughlin	B	Lapdun	B
Land, Drained	B	Las Animas	C	Lawyer	B	Leland	D	Library	D	Laumala	D	Laugenour, Loamy	C
Landavaso	B	Las Aores	D	Lax	C	Lemha	A	Libuse	C	Laurel	B	Substratum	
Landco	C	Las Lucas	B	Laxal	B	Lembos	C	Licha	B	Laurelwood	B	Laugenour, Silty	B
Lander	C	Las Posas	C	Laxton	C	Lemco	C	Lick	B	Lauren	D	Lecrag	D
Landes	B	Las Vegas	D	Laycock	B	Lemert	D	Lickdale	D	Laurentzen	B	Ledford	B
Landlow	C	Lasa	A	Layolnt	C	Lemeta	D	licking	C	Lavacreek	B	Ledgefork	A
Landman	B	Lasalle	D	Layton	A	Leming	C	Lickskillet	D	Lavallee	B	Ledmount	D
Landsend	C	Lasausas	D	Layvlew	D	Lemitar	D	Lidan	C	Lavate	C	Ledow	B
Lane	C	Lasco	B	Lazak	D	Lemm	B	Liddell	BID	Laveaga	B	Leslie	D
Lanesboro	C	Lasil	D	Lazear	D	Lemolo	D	Liddleville	B	Laveen	B	Lesen	D
Lanexa	D	Laska	B	Le Bar	B	Lemond	BID	Lidy	B	Laventana	C	Lespate	C
Laney	B	Lassel	C	Le Sueur	B	Lemonex	C	Lieberman	B	Laverkln	B	linker	B

Lang	C	Lassen	D	Lea	C	Lemoore	C	Lien	D	Lavic	C	linkup	D
Langford	C	Lassiter	B	Leader	B	Lempira	B	Liesnol	D	Ledru	B	Linkville	B
Langhel	B	Lastance	B	Leadore	B	Len	C	Ligget	B	Ledub	BID	Linco	B
Langlade	B	Latah	D	Leadpoint	C	Lena	AID	lightning	D	ledwith	D	Lincoln	A
langlois	D	Latah, High	C	Leadvale	C	Lena, Aooded	D	Lignum	C	Lee	D	Lindaas	CID
Langola	B	Rainfall, Drained		Leadville	B	Lenapah	D	Ligon	D	Leebench	C	Levelton, Drained	C
Langrell	B	Latah, Drained	C	Leaf	D	Lenawee	BID	Ligurta	B	Leeds	C	Leverett	C
Langspring	B	Latahco	C	Leafriver	AID	Lenawee, Poned	D	Lihen	A	Leeffield	C	Leviathan	B
Langston	B	Latahco, Wet	D	Leafu	C	Lenberg	C	Lihue	B	Leeko	B	Levy	D
Langtry	D	Latanler	D	Leagueville	BID	Lenep	C	Likes	A	Leeko, Warm	A	Lanver	C
lanier	A	Latch	A	Leaksville	D	Lenoir	D	Lilah	A	Leelanau	D	Laufer	D
Lanlger	B	Latene	B	Leal	B	Lenz	B	Lilbert	B	Leemont	D	Lantz	D
Laniger, Gravelly	C	Lates	C	Lealandc	D	Lenz, Stony	C	Lilboum	B	Leeper	D	Lauderhlll	BID
Lankbush	D	Latex	C	Leanna	C	Lenz, Very Stony	C	Lillngs	B	Leeray	B	Leck Kill	B
Lankln	C	Latham	D	Leanto	D	Lenzburg	B	Lillngton	B	Leesburg	B	Lesho	C
Lanktree	C	Lather	D	Leaps	C	Leo	A	Ullylands	C	Leesville		Limpia	C
Landak	B	Lathrop	B	Leathan	C	Leola	B	Lilten	C	Lester	B	Levelton	D
Lanona	B	Latgo	B	Leatherman	D	Leon	BID	Lily	B	Leswlll	B	Lantry	B
Lansdale	B	latina	D	Leavenworth	C	Leonard	D	Lim	C	Leta	C	Lauderdale	D
Lansdowne	C	latium	D	Leavers	B	Leanardo	B	Lima	B	Letcher	D	Lebsack	C
lansing	B	Laton	D	Leavitt	B	Leonardtown	D	Limber	B	Letha	C	Leshara	B
Lantern	B	Latonia	B	Leavittville	B	Leoni	B	Limekiln	D	Letment	D	Limones	B
lantis	B	Latouche	D	Lebam	B	Leguleu	D	Limerick	C	Letney	A	Levasy	C
Lanton	D	Latour	B	Lebanon	C	Lerdal	C	Umeridge	D	Leton	D		
Lanton, Low	C	Latourell	B	Lebeau	D	Lerdo	C	Limkng	B	Letort	B		
Precipitation		Lattas	D	Lebec	B	Leroy	B	Limon	C	Letri	BID		
Lantonla	B	Latty	D	Lebo	B	Lerrow	C	Limon, Wet	D	Lettla	B		

Table 4. Sample hydrologic soil groups for United States soils. Note: Two hydrologic soil groups such as B/C indicates the drained/undrained situation. Modifiers shown, e.g., bedrock saturation. Refer to a specific soil series phase found in soil map legend.

Rainfall frequency

The rainfall frequency will be determined by the purpose for the runoff estimate. This may be a design specification for a proposed engineering practice or a policy specification for an evaluation. Once the frequency is known, the 24-hour rainfall can be determined in many sites from a predetermined list of rainfall frequency values. It can also be determined by reading a value from a National Weather Service Map. Figure 2 lists the sources of rainfall frequency data for different geographic areas.

Runoff determination

Runoff can be determined from one of several forms of the runoff equation. The equation itself can be used. Graphs of P and Q by CN are available, and, if you like tables, a P - Q - CN table is available. Needless to say, the equation is the most accurate. Some interpolation may be required (graphical or mathematical) for the other methods. The graphs are in Figure 10.1 in NEH, Section 4; Figure 2-1 in TR-55. Figure 3 shows the TR-55 graph. Exhibit 2-7 in the EFM; Table 2-1 in TR-55; and TR-16 are look-up tables. The TR-16 table is the most complete. Table 5 is a sample of a TR-16.

East of 105th meridian

Hershfield, D.M. 1961. Rainfall frequency atlas of the United States for durations from 30 minutes to 24 hours and return periods from 1 to 100 years. U.S. Dep. Commerce, Weather Bureau Tech. Pap. No. 40. Washington, DC. 115p.

West of 105th meridian

Miller, J.F., R.H. Frederick, and R.J. Tracy. 1973. Precipitation - frequency atlas of the Western United States. Vol. I, Montana; Vol. II, Wyoming; Vol. III, Colorado; Vol. IV, New Mexico; Vol. V, Idaho; Vol. VI, Utah; Vol. VII, Nevada; Vol. VIII, Arizona; Vol. IX, Washington; Vol. X, Oregon; Vol. XI, California. U.S. Dep. Commerce, National Weather Service, NOM atlas 2. Silver Spring, MD.

Alaska

Miller, John F., 1963. Probable maximum precipitation and rainfall-frequency data for Alaska for areas to 400 square miles durations to 24 hours and return periods from 1 to 100 years. U.S. Dep. Commerce, Weather Bur. Tech. Pap. No. 47. Washington, DC. 69 p.

Hawaii

Weather Bureau. 1962. Rainfall-frequency atlas of the Hawaiian Islands for areas to 200 square miles, durations to 24 hours and return periods from 1 to 100 years. U.S. Dep. Commerce, Weather Bur. Tech. Pap. No. 43. Washington, DC. 60 p.

Puerto Rico and Virgin Islands

Weather Bureau. 1961. Generalized estimates of probable maximum precipitation and rainfall-frequency data for Puerto Rico and Virgin Islands for areas to 400 acres, durations to 24 hours, and return periods from 1 to 100 years. U.S. Dep. Commerce, Weather Bur. Tech. Pap. No. 42. Washington, DC. 94 p.

Figure 2. Sources of 24 hr. rainfall data

Runoff for Inches of Rainfall (Curve No. 75)										
Rainfall (Inches)	Rainfall (tenths)									
	0.0	0.1	0.2	0.3	0.4	0.5	0.6	0.7	0.8	.09
0								0.00	0.01	0.02
1	.003	0.05	0.07	0.10	0.13	0.17	0.20	0.24	0.29	0.33
2	0.38	0.43	0.48	0.54	0.59	0.65	0.71	0.77	0.83	0.90
3	0.96	1.03	1.10	1.16	1.23	1.30	1.37	1.45	1.52	1.59
4	1.67	1.74	1.82	1.90	1.97	2.05	2.13	2.21	2.29	2.37
5	2.45	2.53	2.61	2.70	2.78	2.86	2.95	3.03	3.11	3.20
6	3.28	3.37	3.46	3.54	3.63	3.71	3.80	3.89	3.98	4.06
7	4.15	4.24	4.33	4.42	4.51	4.59	4.68	4.77	4.86	4.95
8	5.05	5.13	5.22	5.32	5.41	5.50	5.59	5.68	5.77	5.86
9	5.95	6.05	6.14	6.23	6.32	6.42	6.51	6.60	6.69	6.79
10	6.88	6.97	7.07	7.16	7.25	7.35	7.44	7.53	7.63	7.72
11	7.82	7.91	8.00	8.10	8.19	8.29	8.38	8.48	8.57	8.67
12	8.76	8.86	8.95	9.05	9.14	9.24	9.33	9.43	9.52	9.62
13	9.71	9.81	9.90	10.00	10.09	10.19	10.29	10.38	10.48	10.57
14	10.67	10.77	10.86	10.96	11.05	11.15	11.25	11.34	11.44	11.54
15	11.63	11.73	11.82	11.92	12.02	12.11	12.21	12.31	12.40	12.50
16	12.60	12.69	12.79	12.89	12.99	13.08	13.18	13.28	13.37	13.47
17	13.57	13.67	13.76	13.86	13.96	14.05	14.15	14.25	14.35	14.44
18	14.54	14.64	14.74	14.83	14.93	15.03	15.13	15.22	15.32	15.42
19	15.52	15.61	15.71	15.81	15.91	16.00	16.10	16.20	16.30	16.40
20	16.49	16.59	16.69	16.79	16.88	16.98	17.03	17.18	17.28	17.37

Table 5. Solution of runoff equation. Reference: TR-16.

Application Limitations

The runoff equation is a simple and easy-to-use model that requires only a limited amount of data. The simplifying assumptions mentioned above, though, put constraints on the model that reduce its flexibility or utility, or both. Two important limitations are:

Time independence

For a constant CN, a given amount of rainfall produces a set amount of runoff regardless of the rate of rain fall or how fast the soil can infiltrate water. The lumped system does not identify what part of the loss is infiltration.

Fixed initial abstraction

For a constant CN, the soil moisture or soil properties are not used. While these may be factors in initially selecting the CN value, they are not used in the runoff computations. As an example, a selected CN could occur for soils in two different hydrologic soil groups (A and C) by varying land cover. A different runoff pattern should be expected because of the varying infiltration rates, but the amount of rain required before runoff begins will be the same for both. Thus the accuracy of runoff estimates is reduced for small amounts (0.5 in) of runoff.

Summary

The SCS runoff equation is a lumped system parameter model. The CN's that are used in its application are derived from actual rainfall and runoff data for small watersheds. The model requires limited data (land cover, hydrologic soil group, and rainfall amount) and is especially applicable to ungaged areas that are typical in NRCS work. The runoff solution is simple to apply with the three forms of the equation available (mathematic, graph, and table). Two limitations of the equation are that it is time independent and accuracy is reduced for low runoff amounts because of the $I_a = 0.2 S$ assumption.

By now, you should be able to explain the derivation of and use the SCS Runoff Equation. If you feel you do not understand a portion, you should review that section. In addition, you should retain the Study Guide as a reference until you are satisfied that you can use the runoff equations.

When you are satisfied that you have completed this module, complete the Certification of Completion and send it, through channels, to your state training officer.

Activity 1

Before continuing, complete Activity 1. After completing the activity, review the solution near the back of this module to check your answer. When you have finished, return to the Study Guide text.

1. Name the three major components in a water balance.

a.

b.

c.

2. Briefly state the two assumptions made to create the SCS runoff equation from a water balance.

3. Why were curve numbers used in place of the potential maximum retention after runoff begins?

4. List the types of data required to apply the SCS runoff equation.

a.

b.

c.

d.

e.

f.

5. What are the two major limitations on the runoff equation?

a.

b.

Activity 1

1. Name the three major components in a water balance.

- a. *Rainfall*
- b. *Runoff*
- c. *Losses*

2. Briefly state the two assumptions made to create the SCS runoff equation from a water balance.

Water retention and runoff occur in the same proportion.

$$I_a = 0.2 S$$

3. Why were curve numbers used in place of the potential maximum retention after runoff begins?

S has a wide range of values but a narrow practical range, thus several decimal places are required. Curve numbers have a wider practical range than S, so that 2digit integer values can be used.

4. List the types of data required to apply the SCS runoff equation.

- a. *Land cover*
- b. *Soils and hydrologic soil group*
- c. *Drainage area for each land cover and hydrologic soil group*
- d. *Curve number (derived from the above data)*
- e. *Rainfall frequency*
- f. *24 hour rainfall*

5. What are the two major limitations on the runoff equation?

- a. *Time independence*
- b. *I_a is fixed by CN or S*