
CO652.0710 State Supplement

(a) General Information

This part contains technical information required for the design of various irrigation practices. The practices covered herein are primarily engineering practices and are required for the efficient development and operation of irrigation systems.

(b) Tailwater Recovery System - Code 447

(1) Procedure for Design of Irrigation Tailwater Embankment Ponds

Embankment fills made to impound irrigation tailwater will meet the design requirements and specifications listed in the Colorado Standard and Specifications for Pond - Code 378.

In addition to the reasons cited in 652.0705 "Irrigation runoff, tailwater recovery and reuse," tailwater recovery systems may be considered for one or more of the following reasons:

- Section 208 of the Pollution Control act generally treats tailwater as a non-point source of pollution.
- Tailwater and reuse systems are used in areas of deep-well irrigation because they require much less power to pump water from a tailwater pit than from the aquifer.

Storage requirements for irrigation tailwater will be determined using the same criteria as for tailwater pits.

(2) Procedure for Design of Irrigation Tailwater Pits

(i) **Depth**-Design water surface shall be at the flowline elevation of the inlet structure or 1 foot (0.3 m) below the lowest irrigable land elevation adjacent to the pit, whichever is lower. Retrievable tailwater depth shall be a minimum of 5 feet (1.5 m) and a maximum of 12 feet (3.6 m). Excavated depth (below design water surface elevation) shall be design water depth plus 1 foot (0.3 m).

(ii) **Slopes and Top Width**-Excavated and embankment slopes shall not be steeper than 3:1, except 2:1 slopes may be used for the sides of the pit in soils that will be stable at the slope. Slopes will usually not be flatter than 4:1, except that one or both ends may be flattened to 6:1 for ease of construction and cleanout.

Minimum top width of berms and dikes shall be 8 feet (2.4 m).

(iii) **Mechanical Inlet Structures**-Inlet structures shall be provided to convey the tailwater and/or storm runoff into the pit without erosion damage to the entrance channel or sides of the pit. These structures may consist of chutes, drop structures, or pipes (minimum size - 10 inches [25 cm] in diameter) using corrugated metal, welded steel, plastic, concrete, or other approved material. Structures will be designed in accordance with the Colorado Standards and Specifications. The inlet structures must have the capacity to satisfy the operating needs of the system.

(iv) **Emergency Spillway or Storm Bypass**-Pits shall be surrounded by berms and dikes to prevent surface water from entering the pits at points other than the mechanical inlet structure. A storm bypass or emergency spillway shall be provided which will pass the runoff from a 25-year, 24-hour frequency A.M.C. II storm. Tops of the berms and dikes shall be at least 1 foot (0.3 m) above the maximum water surface in the pit or the spillway, whichever is higher, when passing this storm.

(v) Size

Condition A--Pumpback is intermittent. Volume of storage shall be designed on the basis of the following:

Select T_i , which is the irrigation set time in hours. T_i is listed in Column 11, part 4, of the Colorado Irrigation Guide. The T_i selected should represent the most restrictive design condition for the system. For example, if both border and furrow irrigation methods are planned, furrow irrigation with a high water demand crop (such as corn) should be used to select T_i .

The number of sets (N) to be irrigated with pumpback water per pumpback cycle is usually 2. The maximum number is 2 and the minimum is 1.

Determine whether or not tailwater from the pumpback pump also enters the pit.

T_p is the pumpback time in hours, $T_p = N \times T_i$.

Discharge rate (q) in gallons per minute of the pumpback pump is determined by multiplying the primary source (Q) in gallons per minute by a variable factor (C_a). Values of C_a vary according to soil intake families. Table CO7-1 below lists the values of C_a for the various intake families.

Table CO7-1

Intake Family	Ca Factors		
	Maximum	Median	Minimum
0.1 to 0.3	0.80	0.60	0.40
0.5	0.60	0.46	0.33
1.0 to 3.0	0.40	0.30	0.20

Values of q may be adjusted ± 10 percent if needed to fit the pump discharge rates of commercial suppliers.

The rate of tailwater flow into the collection pits is based on the intake rates of the contributing areas along with the discharge rates of wells, turn outs, and pumps delivering the irrigation water. Use a decimal percentage (%) of Q returning as tailwater from the percentages shown in Table CO7-2 for the soil intake families listed.

Table CO7-2

Intake Family	Percentage (% or %*) of Primary (Q) or Pumpback (q) Flow Delivered as Tailwater
0.1 to 0.3	20
0.5	15
1.0 to 3.0	10

Also select from Table CO7-2 a decimal percentage (%*) of pumpback flow q returning as tailwater.

Find the total of the average field tailwater from the primary source (% Q) and the average tailwater from the pumpback flow (%* q) or (% Q + % q). This is the total tailwater inflow into the pit.

To keep the tailwater system in equilibrium, the pumpback rate q of the pumpback pump minus the total inflow (% Q + % q) must equal the volume of the tailwater pit (V_s) in acre-inches,

or storage = (outflow - inflow)

or $V_s = \frac{q - (\%Q + \%q)}{450} \times T_p = \text{acre-inches}$

Example:

Given: Richfield silt loam, intake family = 0.3
 $Q = 1,000$ gpm Slope Group = 0.2%
 $T_i = 16.3$ hrs. $N = 2$

Find: T_p , q , % Q , %* q , and V_s

$T_p = N \times T_i = 2 \times 16.3 = 32.6$ hrs.

From Table KS7-1, $C_a = 0.6$ (median value)
 $q = C_a \times Q = 0.6 \times 1,000 = 600$ gpm

From Table CO7-2, % = 20%, %* = 20%,
then %Q = 0.20 x 1,000 = 200 gpm,
%*q = 0.20 x 600 = 120 gpm

$$V_s = \frac{q - (\%Q + \%*q)}{450} \times T_p$$

$$V_s = \frac{600 - (200 + 120)}{450} \times 32.6$$

$$= 20.3 \text{ acre-inches}$$

Condition B--Where continuous pumping is desired and the inflow is sufficient both in rate and duration to sustain such a system, the sump (as a minimum) needs to be only large enough to accommodate the pumping.

Pumpback (once started) is continuous as long as the irrigation continues uninterrupted. The maximum value of N shall be one (1). Pumpback rate (q) and the volume of storage (Vs) are determined as shown below.

The following equations may be used to determine the pumpback rate (q) in gallons per minute:

$$q = \frac{\% \times Q}{1 - \%*}, \text{ where } \frac{\%}{1 - \%*} = C_b$$

or

$$q = C_b \times Q$$

Q, q, %, and %* are the same units as used for Condition A.

Cb factors used to determine q are found in Table KS7-3.

Table CO7-3 Cb Factors

Estimated Decimal % of Discharge Q Flowing into the Pit											
0.10				0.15				0.20			
Estimated Decimal % of Pumpback Rate q Flowing into the Pit											
None	0.10	0.15	0.20	None	0.10	0.15	0.20	None	0.10	0.15	0.20
Pumpback Factor Cb											
0.100	0.111	0.118	0.125	0.150	0.167	0.177	0.188	0.200	0.222	0.236	0.250

Volume of tailwater storage (Vs) needed is determined using the following equation:

$$V_s = \frac{(\%Q) \times T_i}{450} = \text{acre-inches}$$

Example:

Given: Q = 1,000 gpm, Ti = 16.3 hrs., % = 0.20,
%* = 0.20

Find: Cb, q, and Vs

$$C_b = \frac{\%}{1 - \%*} = \frac{0.20}{1 - 0.20} = 0.25$$

or refer to Table KS7-3 to find Cb

$$q = C_b \times Q = 0.25 \times 1,000 = 250 \text{ gpm}$$

$$V_s = \frac{(0.20 \times 1,000) \times 16.3}{450} \\ = 7.2 \text{ acre-inches}$$

(vi) Excavated Volume--Approximate excavated volume (Ve) may be found as follows:

Ve = 175 x Vs (ac. in.) = cubic yards cu. yd.)
where 175 is the conversion from ac. in. to cu. yd. with the excavated soil density at 77%.

Condition C--To provide for adequate storage (gallons) for intermittent pumpback systems not indicated, use the following equation:

$$\text{Pond Volume (V)} = \frac{Q \times T}{3} \text{ or } V_s = \frac{q \times T_p}{3}$$

Where:

Q = volume of pumpback system (gpm)
T = irrigation set time (hours)

(vii) Figuring Length, Width, and Volume for Excavated Pits

Volume Equation is: $V = \frac{d}{162} \times (A_1 + 4M + A_2)$

Where:

d = depth (feet)

A₁ = top area (ft²)

M = medium area (ft²)

A₂ = bottom area (ft²)

162 = 27 ft³/yd³ x 6 (sum of areas)

Computing length of pit bottom for desired volume, depth, and width:

Where:

l = bottom length of the pit (feet)

V = Volume (cubic yards)

w = bottom width (feet)

d = depth (feet)

Table CO7-4 Equation for Length at Bottom of Pit

Side Slope	End Slope	Equation for Bottom Length of Pit (l)
3:1	3:1	$\frac{27V - 12d^3 - 3d^2w}{3d^2 + dw}$
2:1	4:1	$\frac{81V - 32d^3 - 12d^2w}{6d^2 + 3dw}$
3:1	4:1	$\frac{27V - 16d^3 - 4d^2w}{3d^2 + dw}$
4:1	4:1	$\frac{81V - 64d^3 - 12d^2w}{12d^2 + 3dw}$
3:1	6:1 - 4:1	$\frac{27V - 20d^3 - 5d^2w}{3d^2 + dw}$

Example:

Given: Storage volume (Vs) of a pit = 20.3 ac-inches
Side Slopes = 3:1, End Slopes = 6:1 - 4:1
Depth = 8', Bottom Width = 50'

Find: Estimated excavation volume (Ve), bottom length, and actual excavated volume

$$V_e = 175 \times V_s$$

$$V_e = 175 \times 20.3 = 3,552.5, \text{ use } 3,553 \text{ cu. yds.}$$

From Table CO7-4, for w = 50', d = 8',
3:1 side slope and 6:1-4:1 end slope

$$L = \frac{27V - 20d^3 - 5d^2w}{3d^2 + dw}$$

$$L = \frac{(27 \times 3553) - (20 \times 8^3) - (5 \times 8^2 \times 50)}{(3 \times 8^2) + (8 \times 50)}$$

$$= \frac{(95,931) - (10,240) - (16,000)}{(192) + (400)}$$

$$= \frac{69,691}{592} = 117.7, \text{ use } 118 \text{ feet}$$

Actual excavated volume:

$$V_e = \frac{d}{162} \times (A_1 + (4 \times M) + A_2)$$

$$= \frac{8}{162} (198 \times 98 + 4 (158 \times 74) + 50 \times 118)$$

$$= \frac{8}{162} (72,072) = 3,559 \text{ cu. yds.}$$

(c) Irrigation Water Conveyance, Pipeline - Codes 430AA Through 430HH, 431

This section summarizes design criteria for irrigation pipelines and includes various technical aids used in pipeline design. For detailed information, refer to the following standards and specifications contained in the Colorado Field Office Technical Guide, Section IV.

- Irrigation Water Conveyance, Aluminum Tubing Pipeline - Code 430AA
- Irrigation Water Conveyance, Asbestos-Cement Pipeline - Code 430BB
- Irrigation Water Conveyance, Non-reinforced Concrete Pipeline - Code 430CC (includes rubber gasket joint, mortar joint, and cast-in-place pipe)
- Irrigation Water Conveyance, High-Pressure, Underground, Plastic Pipeline - Code 431 (includes PVC, ABS, and PE plastic pipe)
- Irrigation Water Conveyance, Low-Pressure, Underground, Plastic Pipeline - Code 430EE (includes PVC, ABS, and PE plastic pipe)
- Irrigation Water Conveyance, Steel Pipeline - Code 430FF
- Irrigation Water Conveyance, Reinforced Plastic Mortar Pipeline - Code 430GG
- Irrigation Water Conveyance, Rigid Gated Pipeline - Code 430HH (includes Aluminum and PVC gated pipe)

(1) Design Criteria Summary

(i) **Working Pressure**--The maximum allowable working pressure is that pressure at which the pipe is warranted to operate continuously without failure. All pipelines should be designed to operate at a pressure less than or equal to the working pressure. Working pressure varies greatly from one pipe material to another; refer to the specific pipeline standard for detailed criteria.

(ii) **External Load Limit**--This is the maximum allowable load due to soil and vehicle weight that may

be applied to the pipe. This factor is usually controlled by placing limits on the maximum and minimum amount of fill that may be placed over a pipeline.

(iii) **Capacity**--The pipeline shall be designed to have sufficient flow capacity to deliver the amount of water necessary to meet planned irrigation requirements. Where water is supplied by a well, the pipeline is generally designed to carry the total well output.

(iv) **Velocity**--Some pipe materials have a maximum velocity limitation. This limitation is usually imposed to minimize damage from surge pressures. The following standards contain velocity limitations:

- 430BB, 430EE, 430GG, 431 - 5 ft/sec maximum
- 430HH - 7 ft/sec maximum

(v) **Friction Losses**--This section indicates the minimum allowable roughness coefficient to be used in calculating pressure or head loss in a pipeline due to friction.

(vi) **Outlets**--These are appurtenances to the pipeline that deliver water to a field or another water conveyance or storage area. Typical examples include line gate valves, risers and alfalfa valves, and hydrants.

(vii) **Check Valves**--These are devices that prevent reverse flow back into a pump and/or well after the pump is shut down. Check valves help to prevent pump damage and help minimize the chance of polluted water in the pipeline flowing back into the well. Check valves are required on all NRCS jobs.

(viii) **Stands**--A stand is a device used to convey water from the source of supply into the pipeline. In most cases, the stand is installed between the pump discharge and the beginning of the pipeline. Stands are generally constructed of steel and aid in controlling air entrapment, surge pressure, and vacuum or negative pressure.

(ix) **Vents**--A vent is a vertical conduit attached to the pipeline that is open on one end to the atmosphere. The purpose of a vent is to allow for overflow when pressure exceeds maximum allowable working

pressure. Vents are rarely used at this time, having been replaced by pressure relief valves.

(x) Pressure Relief Valves--These valves are set to overflow when a specified pressure is reached. This valve protects the pipeline from rupture due to excessive pressure.

Pipe materials conforming to 430AA, 430BB, 430FF, 431 - Pressure relief valves shall be no smaller than 1/4 the nominal pipeline diameter and shall be set to open at a pressure no greater than 5 psi above the pressure rating of the pipe.

Pipe materials conforming to 430CC and 430EE- Pressure relief valves shall have the capacity to discharge the design flow rate of the pipeline at a pressure no greater than 50% more than the working pressure of the pipe. The valves should be set to open at the pipe working pressure.

Pipe materials conforming to 430GG shall conform to both of the above-stated criteria-

Pressure relief valves are to be installed on all pump stands and at the end of the pipeline if there is no valve installed there. On a system that does not have a pump stand, a pressure relief valve shall be installed between the check valve and the beginning of the pipeline.

(xi) Air and Vacuum Release Valves-These valves allow air to escape during pipeline filling and allow air to enter the line to prevent vacuum formation during draining operations. Air and vacuum release valves are to be installed at the following points:

- On the pump stand
- At any summit in the pipeline
- At the end of the pipeline
- On both sides of any in-line control device, such as a line gate valve or hub end gate

- At any location where there are changes in pipeline grade in a downward direction of flow greater than 10 degrees (18%)

Refer to the specific standard for criteria regarding size requirements for air and vacuum release valves.

(xii) Drainage--Provisions for draining the pipeline shall be made if recommended by the manufacturer or required to prevent damage from freezing.

(xiii) Flushing--A valve shall be installed at the end of the pipeline to allow for flushing if a sediment accumulation hazard exists.

(xiv) Thrust Control--Thrust blocks or anchors are to be installed wherever there is an abrupt change in pipeline grade, alignment, or reduction in pipe size. Thrust blocks absorb the axial thrust at these locations, thereby preventing rupture or separation of the pipeline.

(xv) Materials Protection--Certain pipe materials require protective measures to prevent early failure from chemical attack. Aluminum and steel require protection from corrosion. Sulfates and salts may degrade concrete pipe. These substances may occur in irrigation water, the soil in contact with the pipeline, or both.

Refer to Figure CO7-5 for an example of a typical pump installation including appurtenances.

(2) Design Tables

The following design tables may be used in the design of pipeline systems. Included are tables for friction loss in various pipe materials and appurtenances, along with size and capacity guidelines for pressure relief valves and air and vacuum release valves installed on low-pressure plastic pipelines.

For plastic pipelines with working pressures greater than 50 psi, refer to Exhibit 3-7 in the National Engineering Handbook Series Part 650, Engineering Field Handbook, for head loss tables.

Figure CO7-5 Typical Pump Installation

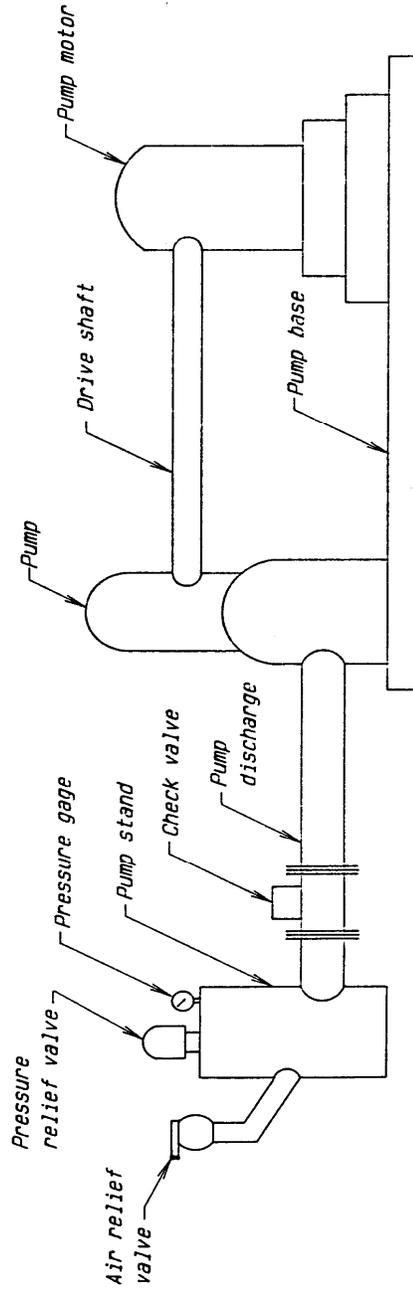


Table CO7-6 Friction Loss in 22 PSI PVC Pipe (ft./1,000 ft.)

FLOW GPM	PIPE SIZE (INCHES)									
	4	6	8	10	12	14	15	18	21	24
100	5.77	0.80	0.20	0.07	0.03	0.01	0.01	0.00	0.00	0.00
200	20.82	2.89	0.71	0.24	0.10	0.05	0.03	0.01	0.01	0.00
300	***	6.12	1.51	0.51	0.21	0.10	0.07	0.03	0.01	0.01
400	***	10.43	2.57	0.87	0.36	0.17	0.12	0.05	0.02	0.01
500	***	15.77	3.88	1.31	0.54	0.25	0.18	0.07	0.03	0.02
600	***	22.10	5.44	1.84	0.76	0.36	0.25	0.10	0.04	0.02
700	***	***	7.24	2.44	1.01	0.47	0.34	0.13	0.06	0.03
800	***	***	9.27	3.13	1.29	0.61	0.43	0.16	0.07	0.04
900	***	***	11.54	3.89	1.60	0.76	0.54	0.21	0.09	0.05
1000	***	***	14.02	4.73	1.95	0.92	0.66	0.25	0.11	0.06
1100	***	***	16.73	5.64	2.32	1.10	0.78	0.30	0.13	0.08
1200	***	***	19.65	6.63	2.73	1.29	0.92	0.35	0.16	0.09
1300	***	***	***	7.69	3.16	1.49	1.07	0.41	0.18	0.10
1400	***	***	***	8.82	3.63	1.71	1.22	0.46	0.21	0.12
1500	***	***	***	10.02	4.12	1.95	1.39	0.53	0.24	0.13
1600	***	***	***	11.29	4.65	2.19	1.57	0.60	0.27	0.15
1700	***	***	***	12.63	5.20	2.45	1.75	0.67	0.30	0.17
1800	***	***	***	14.04	5.78	2.73	1.95	0.74	0.33	0.19
1900	***	***	***	15.52	6.39	3.02	2.15	0.82	0.37	0.21
2000	***	***	***	17.07	7.02	3.32	2.37	0.90	0.40	0.23
2100	***	***	***	18.68	7.69	3.63	2.59	0.98	0.44	0.25
2200	***	***	***	20.37	8.38	3.96	2.83	1.07	0.48	0.27
2300	***	***	***	***	9.10	4.29	3.07	1.17	0.52	0.29
2400	***	***	***	***	9.85	4.65	3.32	1.26	0.57	0.32
2500	***	***	***	***	10.62	5.01	3.58	1.36	0.61	0.34
2600	***	***	***	***	11.42	5.39	3.85	1.46	0.66	0.37
2700	***	***	***	***	12.25	5.78	4.13	1.57	0.70	0.40
2800	***	***	***	***	13.10	6.18	4.42	1.68	0.75	0.42
2900	***	***	***	***	13.98	6.60	4.71	1.79	0.80	0.45
3000	***	***	***	***	14.88	7.02	5.02	1.91	0.86	0.48
3100	***	***	***	***	15.81	7.46	5.33	2.03	0.91	0.51
3200	***	***	***	***	***	7.92	5.66	2.15	0.96	0.54
3300	***	***	***	***	***	8.38	5.99	2.27	1.02	0.57
3400	***	***	***	***	***	8.86	6.33	2.40	1.08	0.61
3500	***	***	***	***	***	9.35	6.68	2.54	1.14	0.64
3600	***	***	***	***	***	9.85	7.04	2.67	1.20	0.68
3700	***	***	***	***	***	10.36	7.40	2.81	1.26	0.71
3800	***	***	***	***	***	10.88	7.78	2.95	1.33	0.75
3900	***	***	***	***	***	11.42	8.16	3.10	1.39	0.78
4000	***	***	***	***	***	11.97	8.55	3.25	1.46	0.82
	4	6	8	10	12	14	15	18.3	21.57	24.27

(ACTUAL I.D. OF PIPE)

BASED ON HAZEN-WILLIAMS EQUATION

C = 150

(Flow rates below this line exceed 5.0 fps maximum)

Table CO7-7 Friction Loss in 50 PSI PVC Pipe (ft./1,000 ft.)

FLOW GPM	PIPE SIZE (INCHES)									
	4	6	8	10	12	14	15	18	21	24
100	5.77	0.81	0.20	0.07	0.03	0.01	0.01	0.00	0.00	0.00
200	20.82	2.92	0.73	0.25	0.10	0.05	0.03	0.01	0.01	0.00
300	44.12	6.18	1.55	0.52	0.21	0.10	0.07	0.03	0.01	0.01
400	***	10.53	2.64	0.89	0.37	0.17	0.12	0.05	0.02	0.01
500	***	15.92	3.99	1.34	0.55	0.26	0.19	0.08	0.03	0.02
600	***	22.32	5.59	1.88	0.77	0.36	0.26	0.11	0.05	0.03
700	***	***	7.43	2.51	1.03	0.48	0.35	0.14	0.06	0.04
800	***	***	9.52	3.21	1.32	0.61	0.45	0.18	0.08	0.05
900	***	***	11.83	3.99	1.64	0.76	0.55	0.22	0.10	0.06
1000	***	***	14.38	4.85	2.00	0.93	0.67	0.27	0.12	0.07
1100	***	***	17.16	5.79	2.38	1.11	0.80	0.33	0.15	0.08
1200	***	***	20.16	6.80	2.80	1.30	0.94	0.38	0.17	0.10
1300	***	***	***	7.89	3.24	1.51	1.09	0.44	0.20	0.11
1400	***	***	***	9.05	3.72	1.73	1.26	0.51	0.23	0.13
1500	***	***	***	10.28	4.23	1.97	1.43	0.58	0.26	0.15
1600	***	***	***	11.58	4.77	2.22	1.61	0.65	0.29	0.16
1700	***	***	***	12.96	5.33	2.48	1.80	0.73	0.33	0.18
1800	***	***	***	14.41	5.93	2.76	2.00	0.81	0.36	0.20
1900	***	***	***	15.92	6.55	3.05	2.21	0.89	0.40	0.23
2000	***	***	***	17.51	7.20	3.35	2.43	0.98	0.44	0.25
2100	***	***	***	19.17	7.88	3.67	2.66	1.08	0.48	0.27
2200	***	***	***	***	8.59	4.00	2.90	1.17	0.53	0.30
2300	***	***	***	***	9.33	4.35	3.15	1.27	0.57	0.32
2400	***	***	***	***	10.10	4.70	3.41	1.38	0.62	0.35
2500	***	***	***	***	10.89	5.07	3.67	1.49	0.67	0.38
2600	***	***	***	***	11.71	5.45	3.95	1.60	0.72	0.40
2700	***	***	***	***	12.56	5.85	4.24	1.71	0.77	0.43
2800	***	***	***	***	13.43	6.26	4.53	1.83	0.82	0.46
2900	***	***	***	***	14.33	6.68	4.84	1.96	0.88	0.49
3000	***	***	***	***	15.26	7.11	5.15	2.08	0.93	0.53
3100	***	***	***	***	16.22	7.55	5.47	2.21	0.99	0.56
3200	***	***	***	***	***	8.01	5.80	2.35	1.05	0.59
3300	***	***	***	***	***	8.48	6.14	2.49	1.12	0.63
3400	***	***	***	***	***	8.96	6.49	2.63	1.18	0.66
3500	***	***	***	***	***	9.46	6.85	2.77	1.24	0.70
3600	***	***	***	***	***	9.96	7.22	2.92	1.31	0.74
3700	***	***	***	***	***	10.48	7.59	3.07	1.38	0.78
3800	***	***	***	***	***	11.01	7.98	3.23	1.45	0.82
3900	***	***	***	***	***	11.56	8.37	3.39	1.52	0.86
4000	***	***	***	***	***	12.11	8.77	3.55	1.59	0.90
	4	5.988	7.958	9.948	11.94	13.93	14.92	18.24	21.18	23.83

(ACTUAL I.D. OF PIPE)

BASED ON HAZEN-WILLIAMS EQUATION

C = 150

NOTE: Pipe (15-inch and greater) only comes in 80 psi
 (Flow rates below this line exceed 5.0 fps maximum)

Table CO7-8 Friction Loss in Gated PVC Pipe (ft./1,000 ft.)

FLOW GPM	PIPE SIZE (INCHES)			
	6	8	10	12
100	0.98	0.23	0.07	0.03
200	3.53	0.83	0.27	0.11
300	7.47	1.75	0.57	0.23
400	12.73	2.98	0.98	0.39
500	19.24	4.51	1.47	0.59
600	26.96	6.31	2.07	0.83
700	35.87	8.40	2.75	1.11
800	***	10.76	3.52	1.42
900	***	13.38	4.38	1.77
1000	***	16.26	5.32	2.15
1100	***	19.40	6.35	2.56
1200	***	22.79	7.46	3.01
1300	***	26.44	8.65	3.49
1400	***	30.33	9.93	4.00
1500	***	***	11.28	4.55
1600	***	***	12.71	5.13
1700	***	***	14.22	5.74
1800	***	***	15.81	6.38
1900	***	***	17.47	7.05
2000	***	***	19.21	7.75
2100	***	***	21.03	8.48
2200	***	***	22.92	9.25
2300	***	***	24.89	10.04
2400	***	***	26.93	10.86
2500	***	***	29.05	11.72
2600	***	***	31.23	12.60
2700	***	***	***	13.51
2800	***	***	***	14.45
2900	***	***	***	15.42
3000	***	***	***	16.42
	5.76	7.76	9.76	11.76

(ACTUAL I.D. OF PIPE)

BASED ON HAZEN-WILLIAMS EQUATION

C = 150

(Flow rates below this line exceed 7.0 fps maximum)

Table CO7-9 Friction Loss in Gated Aluminum Pipe (ft./1,000 ft.)

FLOW GPM	PIPE SIZE (INCHES)				
	4	6	8	10	12
100	8.51	1.13	0.27	0.09	0.04
200	30.70	4.09	0.99	0.33	0.14
300	65.05	8.66	2.09	0.70	0.29
400	***	14.76	3.56	1.19	0.49
500	***	22.31	5.38	1.79	0.74
600	***	31.27	7.54	2.51	1.03
700	***	41.60	10.04	3.34	1.37
800	***	***	12.85	4.28	1.76
900	***	***	15.99	5.33	2.19
1000	***	***	19.43	6.47	2.66
1100	***	***	23.18	7.72	3.17
1200	***	***	27.23	9.07	3.73
1300	***	***	31.58	10.52	4.32
1400	***	***	36.23	12.07	4.96
1500	***	***	41.17	13.72	5.63
1600	***	***	***	15.46	6.35
1700	***	***	***	17.29	7.10
1800	***	***	***	19.22	7.90
1900	***	***	***	21.25	8.73
2000	***	***	***	23.37	9.60
2100	***	***	***	25.58	10.51
2200	***	***	***	27.88	11.45
2300	***	***	***	30.27	12.43
2400	***	***	***	32.75	13.45
2500	***	***	***	35.32	14.51
2600	***	***	***	37.98	15.60
2700	***	***	***	***	16.73
2800	***	***	***	***	17.90
2900	***	***	***	***	19.10
3000	***	***	***	***	20.34
	3.9	5.9	7.9	9.9	11.884

(ACTUAL I.D. OF PIPE)

BASED ON HAZEN-WILLIAMS EQUATION

C = 130

(Flow rates below this line exceed 7.0 fps maximum)

Table CO7-10 Head Loss Through Alfalfa Valves and Portable Hydrants

FLOW (GPM)	Valve and Riser				Portable Hydrant			
	Valve Size (in.)				Outlet Size (in.)			
	8	10	12	14	6	8	10	12
400	0.3	0.1	0.1	0.0	0.3	0.1	0.0	0.0
500	0.4	0.2	0.1	0.0	0.5	0.2	0.1	0.0
600	0.6	0.3	0.1	0.1	0.7	0.2	0.1	0.0
700	0.9	0.4	0.2	0.1	1.0	0.3	0.1	0.1
800	1.1	0.5	0.2	0.1	1.3	0.4	0.2	0.1
900	1.4	0.6	0.3	0.2	1.6	0.5	0.2	0.1
1000	1.8	0.7	0.3	0.2	2.0	0.6	0.3	0.1
1100	2.1	0.9	0.4	0.2	2.4	0.8	0.3	0.2
1200	2.5	1.0	0.5	0.3	2.9	0.9	0.4	0.2
1300	3.0	1.2	0.6	0.3	3.4	1.1	0.4	0.2
1400	3.4	1.4	0.7	0.4	3.9	1.2	0.5	0.2
1500	3.9	1.6	0.8	0.4	4.5	1.4	0.6	0.3
1600	4.5	1.8	0.9	0.5	5.1	1.6	0.7	0.3
1700	5.1	2.1	1.0	0.5	5.8	1.8	0.7	0.4
1800	5.7	2.3	1.1	0.6	6.5	2.1	0.8	0.4
1900	6.3	2.6	1.3	0.7	7.2	2.3	0.9	0.5
2000	7.0	2.9	1.4	0.7	8.0	2.5	1.0	0.5
2100	7.7	3.2	1.5	0.8	8.8	2.8	1.1	0.6
2200	8.5	3.5	1.7	0.9	9.7	3.1	1.3	0.6
2300	9.3	3.8	1.8	1.0	10.6	3.4	1.4	0.7
2400	10.1	4.1	2.0	1.1	11.5	3.6	1.5	0.7
2500	11.0	4.5	2.2	1.2	12.5	4.0	1.6	0.8
2600	11.9	4.9	2.3	1.3	13.5	4.3	1.8	0.8
2700	12.8	5.2	2.5	1.4	14.6	4.6	1.9	0.9
2800	13.8	5.6	2.7	1.5	15.7	5.0	2.0	1.0
2900	14.8	6.0	2.9	1.6	16.8	5.3	2.2	1.1
3000	15.8	6.5	3.1	1.7	18.0	5.7	2.3	1.1

(3) Equations used to develop pipe friction loss tables:

Hazen-Williams Equation:

$$h_f = (1000) \left[\frac{(3.552)(Q)}{(C)(d^{2.63})} \right]^{1/0.54}$$

Manning's Equation:

$$h_f = (1000) \left[\frac{(3.631)(Q)(n)}{(d)^{8/3}} \right]^2$$

Where:

 h_f = friction loss, ft./1000 ft.

Q = flow rate, gpm

d = inside pipe diameter, inches

C = Hazen-Williams roughness coefficient

n = Manning's roughness coefficient

(4) Equations used to develop head loss tableFor alfalfa valves and risers: $h = \frac{2.77V^2}{2g}$ For portable hydrants: $h = \frac{V^2}{2g}$

Where:

h = head loss, ft.

V = flow velocity, ft./sec.

g = 32.2 ft./sec.²**(d) Design Summary for Low Pressure, Underground, Plastic Pipeline - Code 430EE**

This summary covers the major criteria for this practice. Refer to the standard and specifications for further details.

(1) Velocity

Flow Rate at Which a Velocity of 5 ft./sec. Occurs

Nominal Pipe Dia. (Inches)	Flow Rate (GPM)
6	441
8	783
10	1224
12	1763
14	2399
15	2754

(2) Stands

Pump stand or "dogleg" to be located at the outlet of the pump discharge pipe. For a closed system, the minimum diameter of stand or "dogleg" is the pipe diameter.

(3) Check Valves

Install check valves between the pump discharge and the stand.

(4) Pressure Relief Valves

Install on the pump stand or level portion of the "dogleg."

Capacity of Various Pressure Relief Valves, in GPM, at 32.5 PSI, Set to Open at 22 PSI*

Valve Size (Inches)	Waterman		Fresno		
	AA-96C	AA-9	AA-6	PR900	PR600
3	675	---	60	630	165
4	1030	---	120	1045	---
6	---	1580	---	1550	---

* From manufacturer's test data. Refer to manufacturer's literature for PRV capacity for pipelines with working pressures greater than 22 psi.

(5) Air and Vacuum Release Valves (Air Relief Valves)

Minimum Size of Air & Vacuum Release Valve Installation*

<u>ARV Size (Inches)</u>	<u>Nominal Pipe Dia. (Inches)</u>
1-1/2	4
2 & 2-1/2	6 & 8
3	10
4	12, 14, 15

* Based on manufacturer's data for Waterman AV-150 and size table contained in 430EE standard. Other valves may be used subject to meeting opening-size requirements contained in the 430EE standard.

Install air and vacuum release valves at the following locations:

1. Pump stand or "dogleg"
2. Any high point (summit) in the pipeline
3. End of pipeline
4. On either side of any in-line control device, such as a hub end gate or line gate valve
5. Wherever there are changes in pipeline grade in a downward direction of flow greater than 10 degrees (18% slope).

(6) Outlets and Appurtenances

Install a sufficient number of risers and alfalfa valves to facilitate operation of gated pipe, irrigation of borders, or to deliver water to other specific points. Install line gate valves or hub end gates as required to manage water delivery in the system.

(e) Irrigation Land Leveling - Code 464

This section summarizes design criteria and methods for irrigation land leveling. For more detailed information, consult the following references:

- Colorado Supplements to this handbook (National Engineering Handbook Part 652, Irrigation Guide)

- Colorado Standard and Specifications for Irrigation Land Leveling - Code 464
- [National Engineering Handbook, Section 15, Irrigation, Chapter 12, Land Leveling](#)

(1) Design Criteria Summary

(i) **Design Data**--The data used for design should be taken from a detailed grid survey.

(ii) **Irrigation System Design and Operation**--The land leveling design must allow for practical and efficient operation and be compatible with the irrigation development plan. Planning and design criteria listed in Part 1 of the Colorado Irrigation Guide is used in plan development and should be reflected in the land leveling design. For designs in areas where there is not significant irrigation experience or where special conditions exist, refer also to Parts 2 and 3 of the Irrigation Guide. These sections will indicate whether the desired crops can be feasibly irrigated and whether the soils are suitable for gravity irrigation. Refer to Part 4 of the Irrigation Guide when establishing field layout. Field dimensions should be compatible with the application rate, length of run, and slope limitations contained in Part 4.

(iii) **Physical Design Criteria**--Refer to the 464 Standard for criteria relating to field grade, drainage, and elevation control for water distribution systems.

(iv) **Design Method**--One of the following methods is generally used to design the leveling plan:

- **Plane Method** - This method results in a field surface with a uniform downfield slope and cross slope. The centroid of the area is first found and a plane is passed through that point at an elevation equal to the average elevation of the field. The plane is then lowered slightly to provide more excavation than fill (i.e. a cut-fill ratio greater than 1.0). Using this method with manual computation is quite tedious and time consuming for large fields. The PC version of the Plane Surface Design computer program may be used to speed computations. This program is best suited to fields with a limited number of fringe areas.

- **Profile Method** - Using this method, the designer works with profiles of grid lines rather than individual grid elevations. This is a trial- and-error method of adjusting grades on plotted profiles until all design criteria are satisfied. The profile grades are then transferred to the grid map, design elevations computed, and the resulting leveling map checked for adherence to forward slope and cross slope criteria. Also, earthwork computations are made to determine if the cut-fill ratio is satisfactory.
- **Plan-Inspection Method** - This is a trial-and-error method that relies heavily upon the knowledge, experience and judgement of the designer. When using this method, contours are generally plotted on a copy of the grid map. Contour interval will depend upon the topography and desired field slopes. On nearly level fields, a contour interval of 0.5 foot may be desirable. For design slopes of 0.5-1.0 percent, contour intervals of 1.0 foot are generally satisfactory. Using the contours as indicators, areas of relatively uniform slope are assigned trial elevations or grades. The contours may also indicate areas where variable grade may be desirable. Where extreme elevation differences occur, contours may be used to guide placement of bench areas. The trial elevations assigned to the grid points are then checked for conformance to grade criteria. Earthwork computations are performed to determine if the cut-fill ratio is satisfactory. Also, extreme values of cut and fill should be inspected to determine if they are feasible, as should haul distances.
- **Contour Adjustment Method** - This method consists of trial-and-error adjustment of contour lines drawn on the grid map. Design elevations at the grid points are determined by interpolating between the design or adjusted contours. Like the other methods, cuts and fills are determined by comparing the design elevations to the original elevations, grades are checked, and earthwork computations are performed. This method is best

adapted to fields where uniform cross slope is feasible, and for contour furrow irrigation systems.

(v) **Earthwork Calculations**--A number of methods may be used to estimate earthwork volumes during the trial-and-error phase of the design. All final design quantities, however, are to be computed using the four-point method. The following explanation of the four-point method was taken from [Chapter 12, Land Leveling, of NEH-15](#).

(2) Design Documentation and Plans

Figure CO7-6 illustrates the format typically used to develop design documentation and plans.

Where:

V = Volume in cubic feet

L = Perpendicular distance between end planes in feet

A₁ = Area of one end plane in square feet

A₂ = Area of other end plane in square feet

A_m = Area in middle section parallel to end planes in square feet

The use of this formula is laborious, and approximate methods are commonly used.

The four-point method--The four-point method is based on the formula:

$$V_c = \frac{L^2 (H_c^2)}{108 (H_c + H_f)}$$

and

$$V_f = \frac{L^2 (H_f^2)}{108 (H_c + H_f)}$$

Where:

V_c = Volume of cut in cubic yards

V_f = Volume of fill in cubic yards

L = Grid spacing in feet

H_c = Sum of cuts on four corners of a grid square in feet

H_f = Sum of fills on four corners of a grid square in feet

Figure CO7-6 Format for Listing Elevations, Cuts, and Fills on Land Leveling Grid Maps**Normal Field Layout**

(100 ft. x 100 ft. grid)

98	2	98	5	97	7	97	1	- Original elevation
98	2	98	0	97	8	97	6	- Planned or design elevation
0	0	-0	5	+0	1	+0	5	- Cut (-) or fill (+) or use C = cut and F = fill
98	1	98	6	97	9	96	9	
98	2	98	0	97	8	97	6	
+0	1	-0	6	-0	1	+0	7	

Optional: Record as-built (final checkout) elevations above original elevations.

Horizontal Bench Line

To denote elevations on either side of a bench boundary shown horizontally on the leveling map:

+0.6	+0.1	+0.7	+1.1	- Cut (-) or fill (+)
98.8	98.6	98.4	98.2	- Planned or design elevation
98.2	98.5	97.7	97.1	- Original elevation
97.8	97.6	97.4	97.2	- Planned or design elevation
-0.4	-0.9	-0.3	+0.1	- Cut (-) or fill (+)

Optional: Record as-built (final checkout) elevations vertically along the side of the planned and original elevations.

Vertical Bench Line

To denote elevations on either side of a bench boundary shown vertically on the leveling map:

98.5	97	7	- Original elevation
98.0	98.0	97.0	- Planned or design elevation
-0.5	+0.3	-0.7	- Cut (-) or fill (+)
98.6	97	9	
97.8	97.8	96.8	
-0.8	-0.1	-1.1	
98.2	96	5	
97.6	97.6	96.6	
-0.6	+1.1	+0.1	

Optional: Record as-built (final checkout) elevations above original elevations.

Color Coding System

The following color coding system has been used to develop land leveling grid maps:

Original elevation - black ink

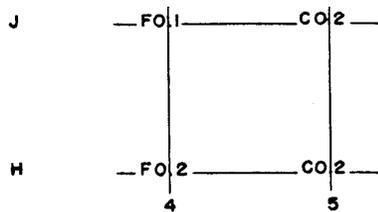
Cut - red

Planned or design elevation - pencil

Fill - blue

As-built or final check elevation - green

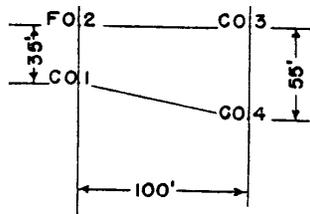
Using the formula, the volume of cut and fill in each grid square can be ascertained and the totals for the field obtained. Table CO7-11 provides a rapid method for determining the excavation and fill in a 100-foot grid square. Let us examine the following grid:



The sum of the cuts are 0.4 and the sum of the fills are 0.3. Since the square is 100 feet to a side, the volume of earthwork as given by Table CO7-11 is as follows:

Excavation	21 cubic yards
Fill	12 cubic yards

The following methods have been found satisfactory for computing the volumes in other than square grids. For grids with four corners:



$$\text{Area grid} = \frac{35 + 55}{2} \times 100 = 4,500 \text{ square feet}$$

$$\text{Area in } 100 \times 100 \text{ grid} = 10,000 \text{ square feet}$$

From Table CO7-11 for a 100 x 100 grid:

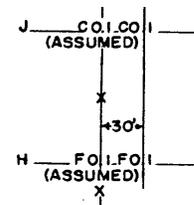
$$\text{Cut} = 59 \text{ cubic yards and fill} = 4 \text{ cubic yards}$$

For the reduced grid:

$$\text{Excavation} = 59 \times \frac{4,500}{10,000} \text{ (or } .45) = 27 \text{ cubic yards}$$

$$\text{Fill} = 4 \times .45 = 2 \text{ cubic yards}$$

Correct use of the four-point method requires that the cut or fill be known at the outside edges of the field. In practice, however, stakes are seldom placed in the fence lines, and usually it is satisfactory to assume that the cut in the fence line is identical to that of the nearest stake. Where abnormal conditions exist, plus stakes should be used. For the grid shown below, the following calculation was made:



$$\text{Area in grid} = 30 \times 100 = 3,000 \text{ square feet}$$

From Table CO7-11:

$$C = 9 \text{ cubic yards}$$

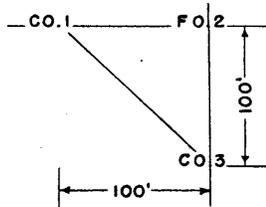
$$F = 9 \text{ cubic yards}$$

$$\text{Corrected Volume} = \frac{3,000}{10,000} \text{ (or } .3) \times 9 = 3 \text{ cubic yards}$$

$$C = 3$$

$$F = 3$$

For a triangular area, the sum of the three corners can be taken and the values in the table reduced to two-thirds. Thus,



Sum of cuts = 0.4

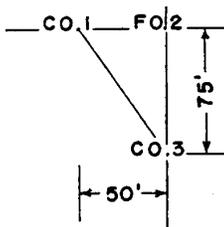
Sum of fills = 0.2

From Table CO7-11, C = 25 and F = 6

Excavation = $\frac{2}{3} \times 25 = 17$ cubic yards

Fill = $\frac{2}{3} \times 6 = 4$ cubic yards

When triangular areas not 100 feet on each side are encountered, the volumes may be determined by further reducing the tabular values in proportion to the product of the length of the sides



Sum of cuts = 0.4

Sum of fills = 0.2

Excavation = $\frac{50 \times 75}{100 \times 100} \times (or .25) \times 25 = 6$ cu. yds.

Fill = $.25 \times 6 = 2$ cubic yards

The four-point method is rapid and gives an accuracy comparable to the accuracy of the original survey.

